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12 substrate. The present paper reports the results of an experimental campaign in which the
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15 strengthened with different types of FRC. Some inclusions have been specifically placed at
16 the interface between the reinforcement and the concrete to simulate adhesion defects. UPV
17 has been applied both in the direct and in the semi-direct mode, to simulate different realistic
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24 **Direct and semi-direct ultrasonic testing for quality control of** 25 **FRC-concrete adhesion**

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29 **Abstract**

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45 Pulse velocity; Direct Transmission; Semi-direct Transmission.

46 **1. Introduction**

47 Advanced Fiber Reinforced Composite (FRC) such as Fiber Reinforced Polymers (FRP) and
48 Fiber Reinforced Cementitious Matrix (FRCM) have been increasingly used in structural
49 engineering for both existing and new buildings strengthening, repairing and retrofitting due
50 to their remarkable strength and stiffness properties, excellent strength-to-weight and stiffness-
51 to-weight ratios, high durability, high corrosion resistance [1-4]. There are many reasons for
52 strengthening and upgrading civil structures: design and construction errors, damage caused
53 by seismic loads and accidental actions, changes in functionality. However, the structural
54 performance is strongly dependent on a proper application, and an almost perfect adhesion
55 between FRC materials and the substrate must be granted to assure the effectiveness of the
56 reinforcement. In light of this, the goal of testing the quality of FRC - substrate adhesion has
57 become of great relevance, and special emphasis has been dedicated to the development of
58 methods for analysing, studying and controlling its performance [5-8].

59 Quality control is an essential aspect not only for verifying the correct setup of fiber reinforced
60 materials, but also for monitoring them during their service life. Indeed, the bond setup in the
61 interface between FRP or FRCM and concrete substrate can be seriously compromised by air
62 presence, inclusions or irregularities, caused by an incorrect application of the reinforced

63 system, or by gaps and voids generated following loads application and exposure to
64 atmospheric agents.

65 The main advantage of Non Destructive Testing (NDT) methods is clearly their ability to non-
66 invasively provide information on adhesion performance, hence in the past various NDT
67 techniques have been proposed with this aim. Among NDTs, Acoustic Methods (AM), based
68 on the analysis of elastic waves propagating through the material, are commonly applied for
69 materials inspection and they are applied also for detecting the presence of subsurface
70 anomalies or defects in elements strengthened with FRC. Bastianini et al. [9] used pulsed echo
71 ultrasonic analysis for testing different type of composite materials applied to different
72 substrates by using the amplitude of the ultrasonic signal. Mahmut et al. [10] performed an
73 acousto-ultrasonic nondestructive testing for detecting and imaging the delamination between
74 carbon fiber-reinforced polymer (CFRP) sheet and concrete substrate. Li et al. [11] used guided
75 waves for identifying CFRP-reinforced concrete debonding. Mirmiran and Wei [12] used
76 ultrasonic pulse velocity testing to quantify the extent and progression of damage in concrete
77 filled FRP-tubes. Mahmoud et al. [13] utilized surface acoustic waves for non-destructive
78 structural health monitoring of concrete specimens externally bonded with CFRP composites
79 and subjected to accelerated aging conditions. Dugnani et al. [14] applied a cross-correlation
80 electromechanical impedance-based approach for detecting adhesive bond-line degradation.
81 La Malfa Ribolla et al. [15, 16] proposed an ultrasonic technique that couples the Akaike
82 information criterion, used as automatic starting time signal detection, and the equivalent time-
83 length of the signal, used as indicator of the energy distribution of the signal with respect to
84 the beginning of the signal, for detecting delamination defects in FRP reinforcement. Concu et
85 al. [17] applied pulse velocity ultrasonic testing for evaluating concrete strengthened with
86 different kinds of FRP.

87 Despite the large amount of contributions and their valuable results, most of the studies on AM
88 for FRC quality control involve the application of sophisticated methodologies, both in terms
89 of instrumentation and, above all, in terms of processing and interpretation of experimental
90 data, therefore the application of these methodologies on site is neither quick and simple nor
91 within the reach of common operators who generally deal with the application and control of
92 FRC. In addition, on-site quality control often requires tools and techniques able to adapt to
93 specific situations, as in the case of inaccessibility of some parts of the element to check -
94 beams, pillars, walls - therefore the study of techniques capable of providing information also
95 in these cases is very useful. Among AM, a technique that meets these requirements is the
96 Ultrasonic Pulse Velocity (UPV) method, a quite simple technique based on the measurement
97 of the velocity of propagation of elastic waves in the material, widely spread and commonly
98 used for on-site materials quality control. Several studies related to the application of UPV to
99 concrete and reinforced concrete diagnostics can be addressed, the main topics being the
100 evaluation of concrete early-stage properties [18-21], the assessment of physical and
101 mechanical properties [22-29], the evaluation of damage [30-37].

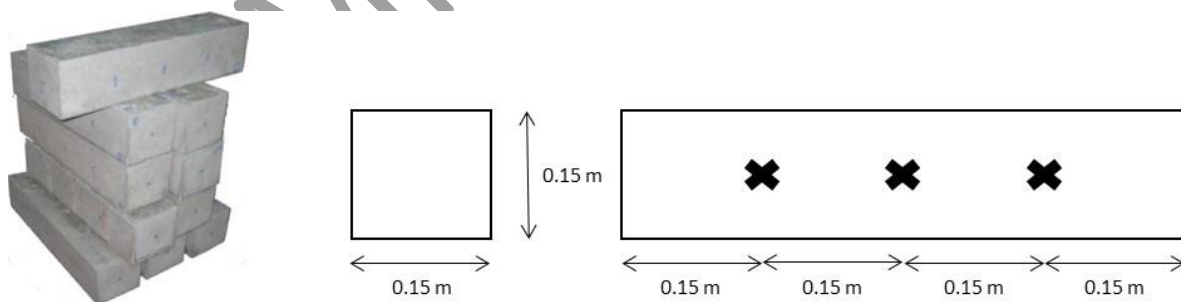
102 UPV can be applied in different ways (direct, semi-direct and non-direct transmission),
103 depending on the possibility to access to the structural element. In particular, the semi-direct
104 mode can be used when the element is accessible only on two adjacent sides, and, although it
105 is normally considered less reliable than the direct mode [38], it can often be the only suitable
106 choice of using UPV on site. As evidence of the interest aroused by the possible use of UPV
107 semi-direct technique in materials diagnostics, the literature presents a number of studies on
108 this topic. Turgut and Kucuk [39] conducted an experimental study to compare direct, non-
109 direct and semi-direct UPV measurements on concrete blocks coming from different mix

110 batches and having different cube compressive strength. Bui et al. [40] applied two types of
 111 configurations, the non-direct and the semi-direct transmission, to cement base samples at
 112 different levels of damage generated by freeze-thaw cycles. Benaicha et al. [41] applied the
 113 direct, semi-direct and non-direct transmission modes to non-destructive evaluation of the
 114 curing degree of fiber-reinforced concrete. Andi et al. [42] analysed the correlation between
 115 UPV direct, semi-direct and non-direct measurements in concrete specimens. Ndagi et al. [43]
 116 reviewed the UPV test by considering different arrangement options and common factors that
 117 affect the results. Ivanchev and Slavchev [44] determined the compressive strength and
 118 modulus of elasticity of concrete using UPV direct, semi-direct and non-direct transmission
 119 ways. Saleem [45] focused on the use of ultrasonic pulse velocity to study the degradation in
 120 steel-concrete bond subjected to increasing loading using both direct and semidirect testing
 121 methods.

122 To the knowledge of the authors, there are no in-depth studies on the application of UPV for
 123 the quality control of the adhesion between FRC and substrate. For this reason, an experimental
 124 test has been run with the purpose of checking the efficacy of UPV in detecting anomalies at
 125 the interface between FRC and concrete. To this end, UPV has been carried out on thirty
 126 concrete specimens strengthened with three different types of FRC. Some inclusions have been
 127 specifically placed at the interface between the reinforcement and the concrete to simulate
 128 adhesion defects. UPV has been applied both in the direct and in the semi-direct mode, to
 129 simulate different realistic on-site situations, including the possible inaccessibility of some
 130 parts of the element to check. Preliminary results point out the possibility to extend the use of
 131 this relatively cheap, fast, and widespread method for quick control of FRC-concrete interface.

132 2. Materials

133 The experimental tests have been carried out on thirty beams 0.15x0.15x0.6m made of concrete
 134 C30/37 [46] (Fig. 1 left). The locations where applying the transducers for UPV, three for each
 135 lateral face of specimens, have been marked (Fig. 1 right). These points lie on the centre line
 136 of each face, are 0.15 m apart from each other and from the edge of the base faces and have
 137 been marked with progressive numbering from 1 to 3 following the same direction for all faces.



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139

Figure 1. Concrete specimens (left). UPV measurements points (right).

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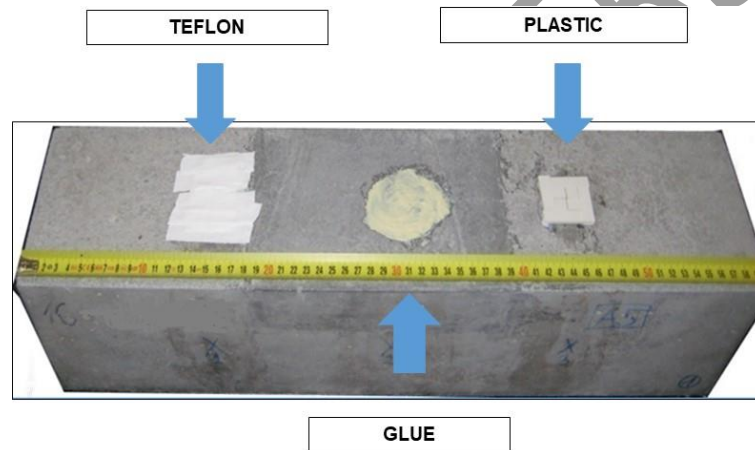
Table 1. Main mechanical properties of the strengthening fibers (from manufacturer).

	Ruredil X Lam 50S	Ruredil X Wrap 310	Ruredil XMesh Gold
Tensile strength (MPa)	>2200	4800	5800
Tensile modulus of elasticity (GPa)	165	240	270

Ultimate elongation (%)	1.3	2	2.15
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141 The strengthening systems were provided by Ruredil and were of three types: FRP lamina,
 142 FRP sheet and FRCM sheet. The physical and mechanical properties of the strengthening fibers
 143 were provide by the manufacturer and are reported in Table 1.

144 Each type of reinforcement was applied on 10 specimens, thus identifying 3 groups of
 145 reinforced specimens called group A, group B, and group C. For each group, perfect adhesion
 146 between the reinforcement and the substrate was sought on two specimens, while adhesion
 147 defects were simulated on the remaining eight specimens, placing on the lateral face, where the
 148 reinforcement was then applied, three types of artificial defects: Teflon element (T),
 149 accumulation of glue (G), and plastic element (P) (see Fig. 2 and Table 2). These defects were
 150 chosen to simulate some common errors in FRC application: Teflon simulates the lack of
 151 adhesion between the FRC and the substrate [11, 15, 16], the accumulation of glue simulates
 152 the variation of the thickness of the glue layer [9], plastic simulates the presence of an inclusion
 153 of a material different from the FRC and the substrate. The defects were glued to concrete by
 154 means of a bicomponent epoxy resin.



155 Figure 2. Artificial adhesion defects: Teflon (left), Glue (centre), Plastic (right).

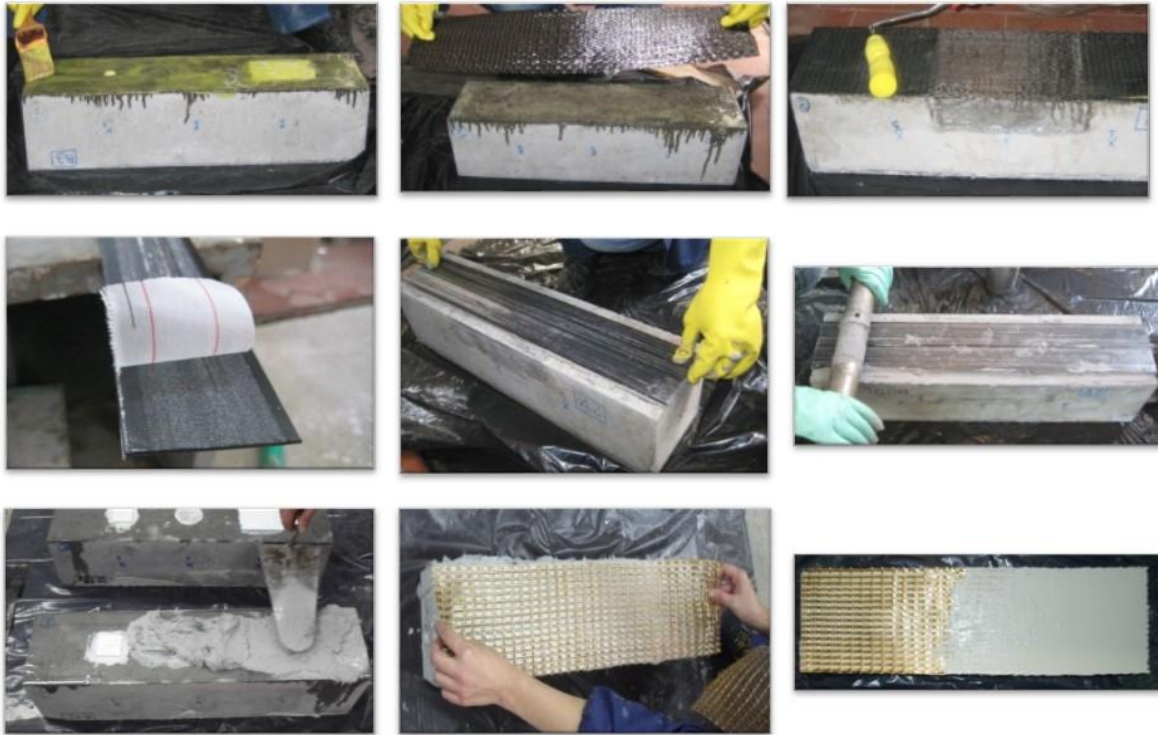
156 Table 2. Defects.

Defect	Shape	Plan size (m)
Teflon (T)	rectangular	0.04x0.06
Glue (G)	round	0.04
Plastic (P)	square	0.03

157 Table 3 shows the schematic of the groups of specimens and the adhesion defects, whereas
 158 Figure 3 shows some steps of the strengthening process. The application of the reinforcement
 159 systems involved different phases: i) homogenization of the concrete substrate, ii) application
 160 of bicomponent epoxy resin, iii) application of the FRC lamina or sheets, iv) impregnation of
 161 the reinforcement system, v) application of a second layer of resin in case of the FRP sheets.

Table 3. Specimens, reinforcements, and defects.

Group	Reinforcement	Specimen	Defect
A	FRP lamina (Ruredil X Lam 50S)	A1, A2	-
		A3-A10	T, G, P
B	FRP sheet (Ruredil X Wrap 310)	B1, B2	-
		B3-B10	T, G, P
C	FRCM sheet (Ruredil XMesh Gold)	C1, C2	-
		C3-C10	T, G, P



163 Figure 3. Strengthening process. FRP lamina (up), FRP sheet (center), FRCM sheet (down).

164 **3. Ultrasonic Pulse Velocity Testing**

165 Ultrasonic testing methods consist in the study of the propagation of elastic waves within the
 166 material. The most widespread way of applying ultrasonic testing for materials diagnostics is
 167 based on the measurement of the velocity V of longitudinal waves propagating through the
 168 material, thus performing the so-called Ultrasonic Pulse Velocity (UPV) testing. A
 169 characteristic of the method is that it considers the overall mechanical properties of the material
 170 [47].

171 **3.1.Theoretical model**

172 When a material is modified by means of an elastic vibration, the perturbation propagates in it
 173 in a certain time in the form of an acoustic wave originating from the vibration of the particles
 174 that make up the material. As for all wave phenomena, it is possible to define the wavelength
 175 λ , the period T , and the frequency f , quantities linked together by the relationship:

$$V = \lambda / T = \lambda \cdot f \quad (1)$$

176 where V represents the velocity of the acoustic wave in the material. When the frequency
177 exceeds the value of 20kHz, the acoustic wave is called ultrasonic wave.

178 The propagation of ultrasonic waves can take place in materials in different ways. Ultrasonic
179 waves can propagate as a result of oscillations of the particles parallel to the direction of the
180 wave front motion (longitudinal waves) or perpendicular to it (transverse waves). These are the
181 waves most frequently used for UPV of on-site structures and construction materials. The
182 ultrasonic waves propagate in the materials under the influence of a local pressure P , which
183 represents the pressure variation the particles undergo when the wave passes, evaluated with
184 respect to a static equilibrium value. According to the pressure variation and the oscillation
185 velocity ξ of the particles, the acoustic impedance Z of the material is defined by the following
186 relationship:

$$Z = P / \xi \quad (2)$$

187 and when the propagation occurs without phase shift between P and ξ , then Z can be expressed
188 as a function of the physical properties of the material by means of the expression:

$$Z = d \cdot V \quad (3)$$

189 where d is the density of the material and V is the propagation velocity of the wave. Z can be
190 considered as a local characteristic of the material and expresses its resistance to wave
191 propagation. When an ultrasonic wave impacts a surface or inhomogeneity, complex changes
192 can occur that modify the motion of the particles due to Z differences of the materials involved.
193 The incident wave undergoes transmission, reflection, and refraction phenomena according to
194 the following values of transmission coefficient t and reflection coefficient r :

$$t = 4Z_2 / (Z_2 + Z_1)^2 \quad (4)$$

$$r = (Z_2 - Z_1) / (Z_2 + Z_1) \quad (5)$$

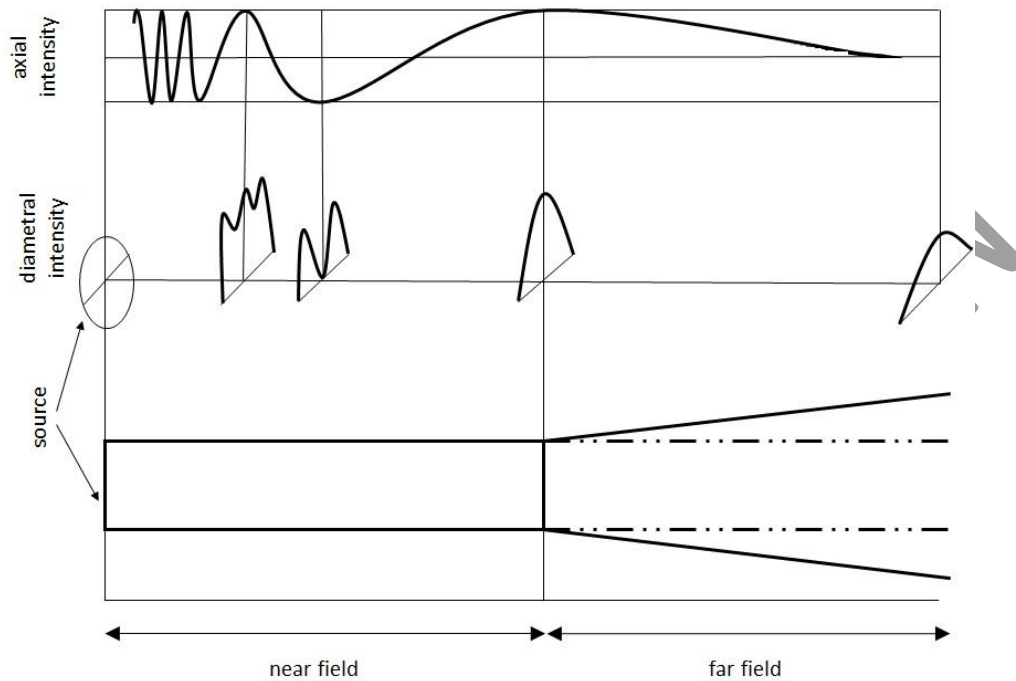
195 where Z_1 and Z_2 are the acoustic impedances of the materials involved. The incident,
196 transmitted, reflected, and refracted waves follow the Snell's law:

$$\sin \alpha_1 / \sin \alpha_2 = V_1 / V_2 \quad (6)$$

197 where α_1 and α_2 identify the direction of the waves propagating in materials 1 and 2, and V_1
198 and V_2 are the velocity of propagation of waves in materials 1 and 2. From the foregoing
199 considerations, it follows that the variation of V can provide indications that the ultrasonic
200 beam has run into discontinuities along its path, therefore V is a parameter widely used in the
201 diagnostics of materials.

202 During the passage through the material, the ultrasonic waves also undergo attenuation
203 phenomena caused essentially by absorption and scattering. Scattering, which occurs whenever
204 waves impact particles of a size comparable with their wavelength, results in the overall
205 deviation of part of the incident waves from the original path. A further cause for energy loss
206 is intrinsic to the way waves are generated by the transducer. In fact, the beam of ultrasonic
207 waves generated by a probe is divergent, and this involves a progressive reduction of the energy

208 associated with the wave as the distance from the source increases. The intensity of the
 209 ultrasonic waves inside the beam generated by the transducer is not constant but varies due to
 210 the finite dimensions of the source which give rise to diffraction phenomena. Although the
 211 representation of the emitted ultrasonic beam is rather complex, a commonly accepted
 212 simplified schematic is shown in Figure 4.



213

214

Figure 4. Emitted ultrasonic beam.

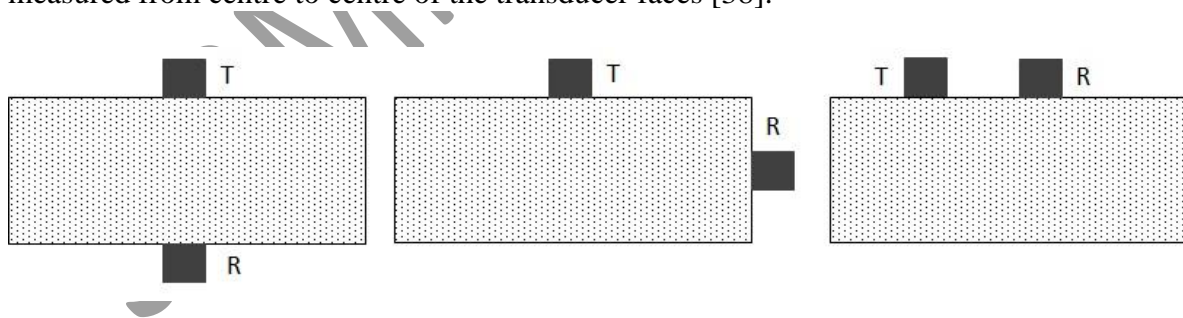
215 Assuming that the transducer is a cylinder with a certain diameter, the ultrasonic beam appears
 216 characterized by a first zone, called near field or Fresnel zone, in which the intensity is
 217 fluctuating between a minimum and a maximum value; the length of the near field increases
 218 with increasing wave frequency and transducer diameter. Beyond the near field, the ultrasonic
 219 beam becomes more uniform, and there is a tendency to diverge according to an angle inversely
 220 proportional to wave frequency and transducer diameter; this zone is called far field or
 221 Fraunhofer zone, and here the ultrasonic energy gradually decreases until it disappears. As
 222 shown in Fig. 4, in the far field there is a decrease in wave energy with increasing distance
 223 from the source, and for a given distance this decrease is minimal in the axial direction of the
 224 ultrasonic beam and increases towards the end of the emission cone, in which it is maximum.
 225 Therefore, if the detection of the waves is made in a different direction from the axial one in
 226 which the energy is maximum, the attenuation can be such as to affect the possibility to
 227 accurately detect the instant of arrival of the wave at the receiver placed at a certain distance
 228 from the source. For this reason, the standards relating to concrete UPV generally suggest the
 229 direct transmission mode, in which the source and the receiver are aligned along the ultrasonic
 230 beam axis and the energy associated with the wave is maximum.

231 When applied to concrete, UPV measures the velocity of propagation of the elastic waves,
 232 determined as the ratio between the distance between transmitter and receiver and the time
 233 taken to travel it according to the following equation [38]:

$$V = L/T \quad (7)$$

234 where V is the wave velocity, L is the path length, T is the time taken by the wave to travel the
 235 path.

236 It should be borne in mind that the elastic waves undergo, within the examined element,
 237 refractions and reflections, due to the presence of aggregates, cracks, voids. This entails an
 238 attenuation of the signal due to the absorption of energy, as previously illustrated. Furthermore,
 239 due to the effect of voids or cracks, the path actually made by the elastic waves can be longer
 240 than the distance between the transmitter and the receiver. The velocity calculated according
 241 to Eq. 7 may differ from the actual propagation velocity through the material. For this reason,
 242 the velocity calculated according to Eq. 7 is often called apparent velocity. As previously
 243 illustrated, although the direction in which the maximum energy propagates is along the
 244 ultrasonic beam axis, it is possible to detect waves which have travelled through the concrete
 245 in some other direction. It is therefore possible to make measurements of wave velocity by
 246 placing the two transducers on opposite faces (direct transmission DT), on adjacent faces
 247 (semi-direct transmission ST), or on the same face (indirect transmission IT) of the element
 248 (Fig. 5). In ST and IT modes, a lower precision than DT mode in the determination of the first
 249 arrival of the wave to the receiver is expected due to the greater attenuation, and this generally
 250 results in velocity values lower than those obtained with the DT. The ST is used when the DT
 251 cannot be used, for example, at the corners of structures, While IT is the least sensitive method,
 252 and should be used when only one face of the concrete is accessible, or when the quality of the
 253 surface concrete relative to the overall quality is of interest [38]. The IT sensitivity to the
 254 conditions of the superficial layer would lead to choose this mode to test the adhesion between
 255 the FRC and the underlying concrete. However, as highlighted by Annex A of [38] the IT
 256 arrangement involves some uncertainty regarding the exact length of the transmission path,
 257 since the areas of contact between transducers and concrete are of significant size, and therefore
 258 requires more complex measurements and data processing than DT and ST, which partially
 259 invalidates its ease of use and expedition. IT therefore belongs to the methods having greater
 260 complexity than those considered in the present paper, so the analysis of its effectiveness is to
 261 be considered out of the scope of this experimentation, in which only DT and ST are
 262 considered. For DT, the path length is the shortest distance between the transducers, whereas
 263 for ST it is generally found to be sufficiently accurate to take the path length as the distance
 264 measured from centre to centre of the transducer faces [38].



265
 266 Figure 5. Direct transmission (left), Semi-direct transmission (centre), Indirect transmission (right). T = emitter
 267 transducer, R = receiver transducer.

268 The velocity V of longitudinal waves depends on the elastic constant of the material concerned
 269 through the following relation [48, 49]:

$$V = \sqrt{\left(\frac{E_d}{d}\right) \cdot \frac{(1 - n)}{(1 + n) \cdot (1 - 2n)}} \quad (8)$$

270 where E_d [$\text{kg}/(\text{m}\cdot\text{s}^2)$] is the dynamic modulus of elasticity, ν is the dynamic Poisson's number,
271 d [kg/m^3] is the density, thus, the velocity V [m/s] can give information on the physical-
272 mechanical condition of the materials. European Standard EN 12504-4 [38] suggests that the
273 pulse velocity can be used for the determination of the uniformity of concrete, the presence of
274 cracks or voids, changes in properties with time and in the determination of dynamic physical
275 properties, and that it may also be used to estimate the strength of concrete elements on site or
276 of specimens, although it is not intended as an alternative to the direct measurement of the
277 concrete compressive strength. Standard [38] also recommends considering the factors that can
278 influence the pulse velocity measurements: i) moisture content of the concrete, mainly
279 dependent on the curing conditions, to be carefully considered when estimating strength; ii)
280 concrete temperature, to be considered if outside the range 10°C - 30°C ; iii) path length over
281 which the pulse velocity is measured, that should be long enough not to be significantly
282 affected by the heterogeneous nature of the concrete; iv) shape and size of the specimen, that
283 can be neglected as influencing factors unless the least dimension of the specimen is less than
284 a minimum value suggested in [38], Annex B, as a function of transducers frequency. All these
285 factors can be considered irrelevant with respect to the experimentation carried out, as the
286 specimens underwent the same curing conditions until hardening, the temperature was within
287 the safe range, the minimum path length (0.15 m) was not less than the limit proposed in [38]
288 and the minimum dimension of the specimen (0.15 m) was higher than the limit (0.065 m)
289 suggested by [38] for the transducers frequency used (54k Hz).

290 3.2.Experimental

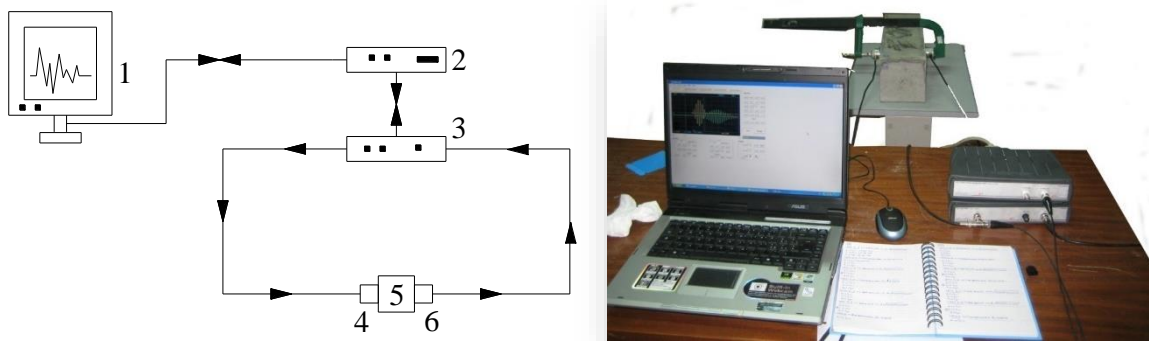
291 The UPV has been carried out on the specimens before and after the reinforcement application
292 applying both DT and ST, to evaluate its reliability in detecting adhesion problems and
293 simulate different testing situations on site.

294 The testing equipment included:

- 295 • a Velleman Instruments arbitrary waveform generator for signal generation,
- 296 • a Velleman Instruments digital oscilloscope for signal visualization and preliminary
297 analysis,
- 298 • a pair of piezoelectric transducers (54 kHz resonant frequency) for signal transmission and
299 acquisition,
- 300 • a PC for data storage and signal processing.

301 The choice of the frequency of the transducers mainly depends on the need to avoid interference
302 from the natural inhomogeneities of the concrete, namely the aggregates. Considering that the
303 maximum diameter of specimens' aggregates was about 0.02 m, a wavelength equal to at least
304 twice the diameter, and therefore equal to about 0.04 m, was chosen. A preliminary test carried
305 out on specimens highlighted an expected propagation velocity of about 2000 m/s on average.
306 By applying Equation 7, in which V is the expected velocity and L is the wavelength, a
307 frequency $f = 1/T$ of approximately 50 kHz is obtained. The 54 kHz frequency was therefore
308 chosen, guaranteed by commercially available transducers.

309 Transparent Vaseline was used to couple transducers to specimens, to reduce signal energy
 310 dissipation due to different acoustic impedance between the material kept in contact. The
 311 measurement set-up and the operative procedure are shown in Figure 6.

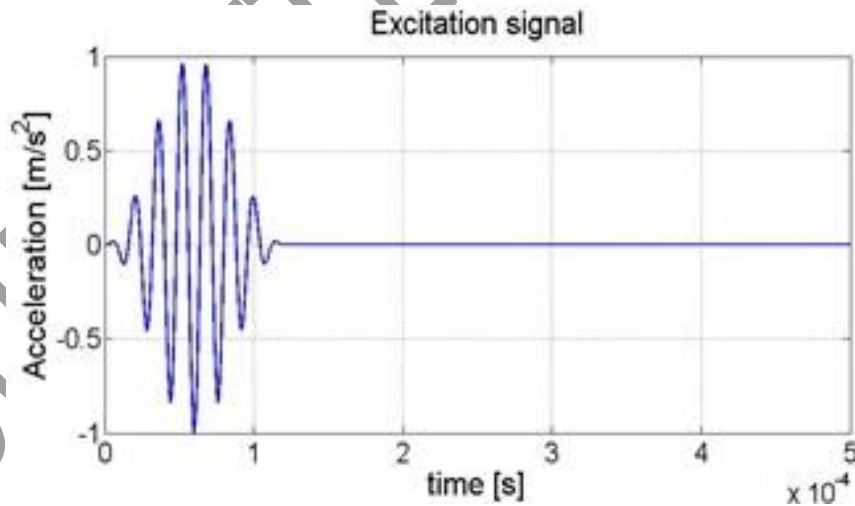


312 Figure 6. UPV set-up. 1) PC, 2) Signal generator, 3) Oscilloscope, 4,6) Transducers, 5) Specimen.

313 The excitation signal was a seven-and half-cycle tone burst enclosed in a Hanning window
 314 (Fig. 7), used to reduce the leakage phenomena in AM [50-52] and described by the following
 315 equation:

$$y(t) = \frac{1}{2} \cdot \sin(2\pi f t) \cdot \left[1 - \cos\left(\frac{2}{15} 2\pi f t\right) \right] \quad (9)$$

316 where f is the frequency of the starting sine wave.

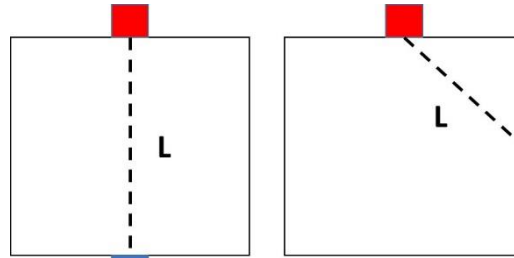


317
 318 Figure 7. Excitation signal.

319 For each specimen, travel time T has been measured along selected paths of length L , different
 320 depending on whether DT or ST was applied (Fig. 8), and then propagation velocity V has been
 321 determined as the ratio L/T , as suggested by [38]. T is taken as the time that elapses between
 322 the first deflection of the trace of the signal emitted and that of the signal received (Fig. 9). The
 323 process of extracting the first arrival travel time of a wave can be cumbersome, especially when
 324 the waveform received is unclear, therefore various signal processing methods are proposed in

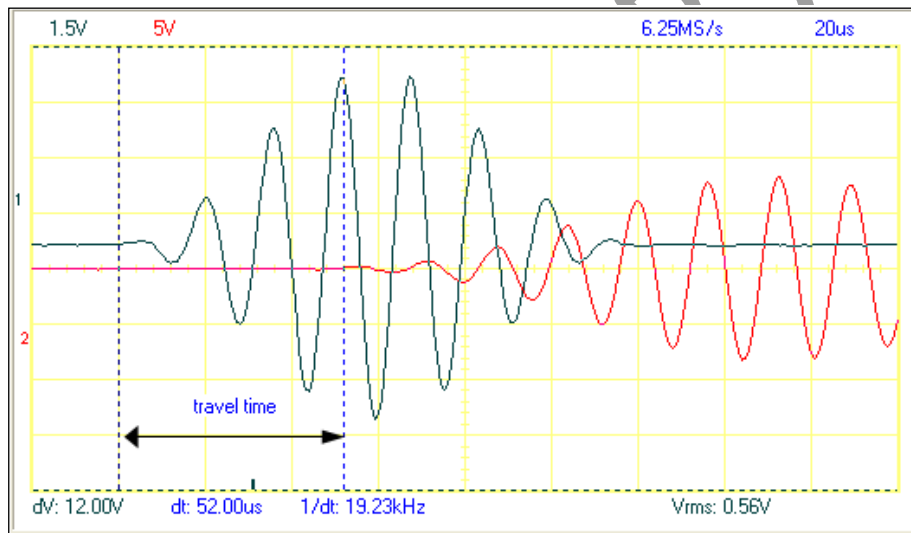
325 the literature to automate the process. However, automatic detection methods can be complex
326 and unsuitable for fast UPV applications, therefore manual picking was used in this
327 experimentation according to Boulay et al. [53], who commented that automatic methods are
328 often ineffective and recommended manual picking.

329



330 Figure 8. Path length L. DT (left), ST (right).

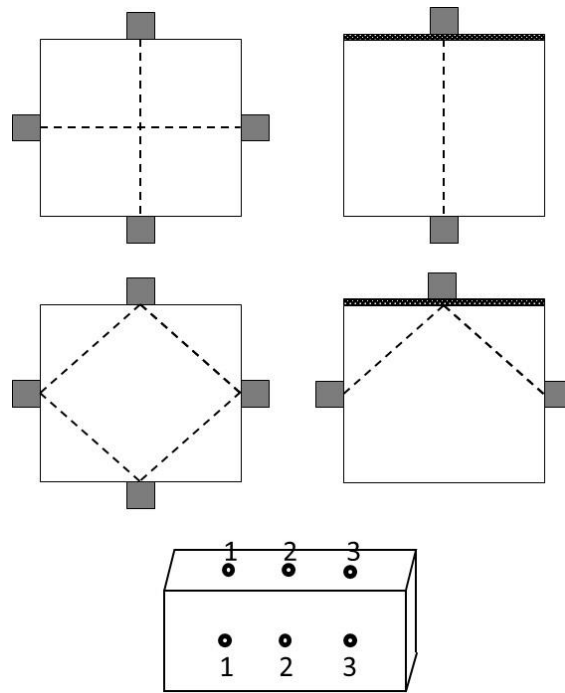
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332

333 Figure 9. Measure of travel time T (emitted signal in green, received signal in red).

334 Figure 10 shows the measuring points and the related hypothetical paths of the signal in the
335 case of DT and ST carried out before and after the application of the reinforcement. It is worth
336 noting that in reinforced specimens the adhesion defects have been settled below the
337 reinforcement in correspondence with the three positions of the emitter transducer, so in the
338 measurements carried out after strengthening one end of the signal path always coincides with
339 the position of the defect.



340

341 Figure 10. Scheme of the UPV measurements. DT (up) and ST (centre) before strengthening (left) and after
 342 strengthening (right). Three measurement points for each lateral face of the specimen (down).

343 4. Results and discussion

344 According to the UPV measurements scheme reported in Figure 10, on each specimen, six DT
 345 measurements and twelve ST measurements of V have been carried out before the application
 346 of the reinforcement, whereas after strengthening three DT measurements and six ST
 347 measurements have been carried out.

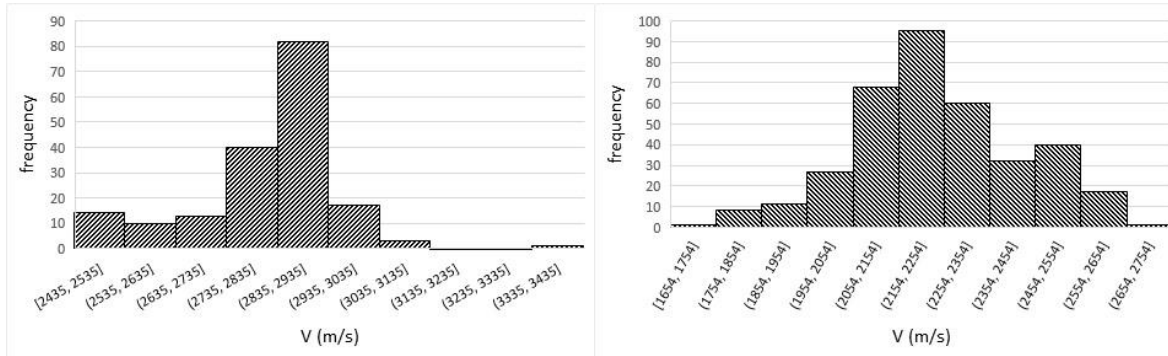
348 4.1. UPV of specimens without adhesion defects

349 Table 4 reports the results of UPV carried out on all specimens before strengthening and on
 350 specimens without artificial adhesion defects after strengthening.

351 Table 4. Velocity of propagation in reference specimens and in non-defective strengthened specimens. s =
 352 number of specimens, n = total number of measurements, V_m = average V, DS = standard deviation, CV =
 353 coeff. of variation, PD = percentage difference between DT and ST values.

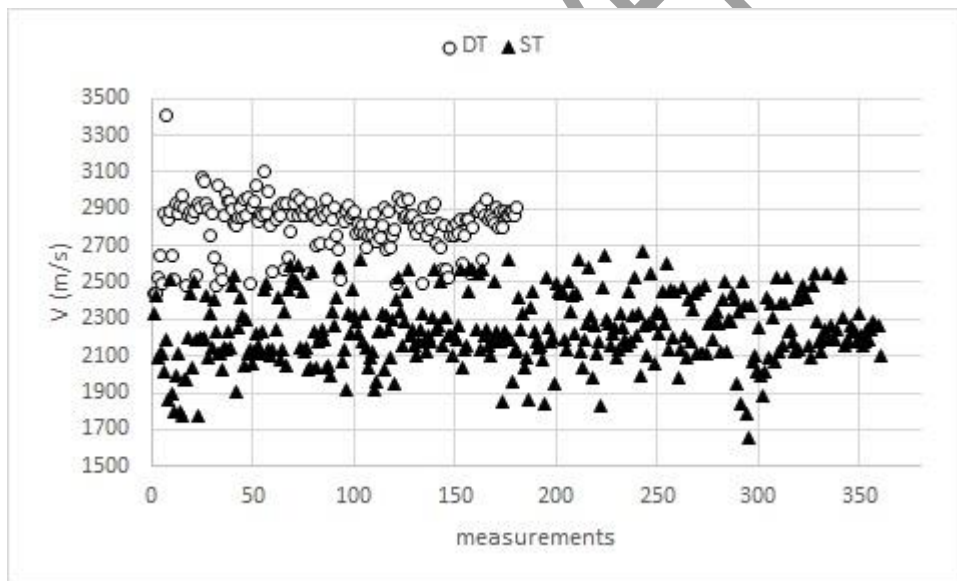
	Before strengthening		FRP lamina strengthening		FRP sheet strengthening		FRCM sheet strengthening	
	DT	ST	DT	ST	DT	ST	DT	ST
s	30	30	2	2	2	2	2	2
n	180	360	6	12	6	12	6	12
V _m (m/s)	2814	2256	2765	2127	2538	2047	2646	2246
DS (m/s)	142	178	54	171	320	218	80	288
CV	0.05	0.08	0.019	0.080	0.126	0.107	0.30	0.128
PD (%)	27.4		29.9		23.9		17.8	

354 Regarding the tests on the specimens before strengthening, it can be observed that the data
 355 acquired via DT and via ST are little scattered, as can be seen both from the CV in Table 4 and
 356 from the distributions in Figure 11. Furthermore, the two sets of data, DT and ST, for which a
 357 percentage difference of about 27% can be calculated, are clearly distinguishable from each
 358 other, as illustrated in the scatter plot in Figure 12.



359
 360 Figure 11. Distribution of V (m/s). DT measurements (left) and ST measurements (right).

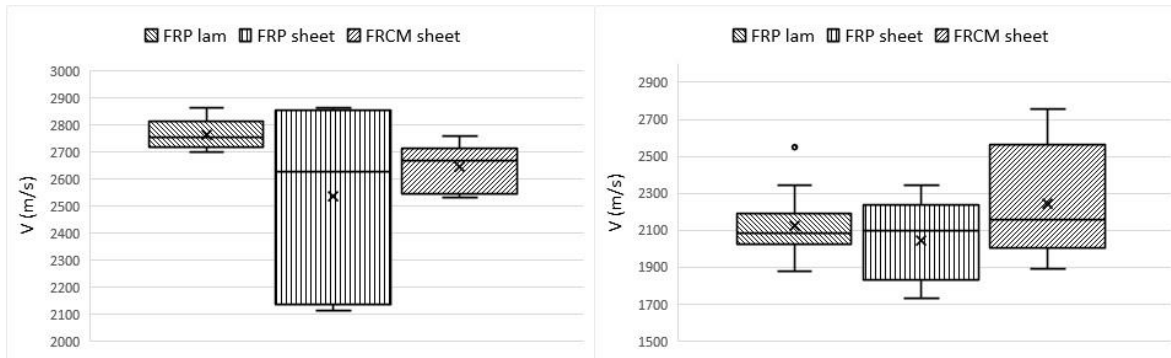
361



362
 363 Figure 12. Scattering of V (m/s) data for DT and ST measurements.

364 This aspect is confirmed by the application of the Mann-Whitney U test, which allows to
 365 evaluate whether the two samples are extracted from one single population (null hypothesis)
 366 or not. Assuming a significance level $p = 0.05$, the Mann-Whitney U test compares the
 367 calculated U to the critical value U^* below which the significance is less than 0.05. The Mann-
 368 Whitney U test applied to the two sets V (DT) and V (ST) gives $U = 688 < U^* = 32400$. This
 369 means that the two data sets are extracted from different populations with a probability higher
 370 than 95% and highlights how the two measurement methods provide results significantly
 371 different. This outcome, in line with literature findings, is consistent with the theoretical model
 372 illustrated in Section 3.1, and in particular with the fact that the increase in attenuation for
 373 measurements made in a direction other than the axial one affects the measurement of waves
 374 transit time and consequently leads to lower waves velocity.

375 Regarding the tests on specimens strengthened with the three types of reinforcement applied
 376 correctly without the insertion of artificial adhesion defects, it can be noted in Table 4 that the
 377 percentage difference between DT and ST measurements varies between about 18% and 30%
 378 depending on the reinforcement type. In the context of each measurement method (DT or ST),
 379 on the other hand, it can be observed that the three reinforcements do not have an appreciably
 380 different effect on V measurements. This can be seen from the plots in Figure 13, which show
 381 a partial overlap between V data, and is confirmed also in this case by the application of the
 382 Mann-Whitney U test, which gives the probability of rejecting the null hypothesis (Table 5).



383

384 Figure 13. V (m/s) data in non-defective strengthened specimens. DT (left) and ST (right).

385 Table 5. Probability of rejecting the null hypothesis of Mann-Whitney U test for specimens with different
 386 reinforcements. Significance level $p = 0.05$. U is compared to critical value U^* at $p < 0.05$. $U^* = 5$ for DT
 387 measurements and $= 37$ for ST measurements.

	FRP lamina vs FRP sheet		FRP lamina vs FRCM sheet		FRP sheet vs FRCM sheet	
	DT	ST	DT	ST	DT	ST
Mann-Whitney U test	15 > 5	61.5 > 37	3.5 < 5	58.5 > 37	18 > 5	47 > 37

388 It can be observed that the significance levels are close to but higher than the chosen limit value
 389 $p = 0.05$ in all cases except one in which U is slightly lower than the critical value. This outcome
 390 means that the three sets of V data (FRP lamina, FRP sheet, FRCM sheet) can be attributed to
 391 different populations with a probability of only 5% in all cases, except the one of FRP lamina
 392 vs FRCM sheet for which this probability is a bit higher. However, in any case, the null
 393 hypothesis, which states that the three samples are extracted from one single population, is
 394 prevalent.

395 **4.2.UPV of specimens with adhesion defects**

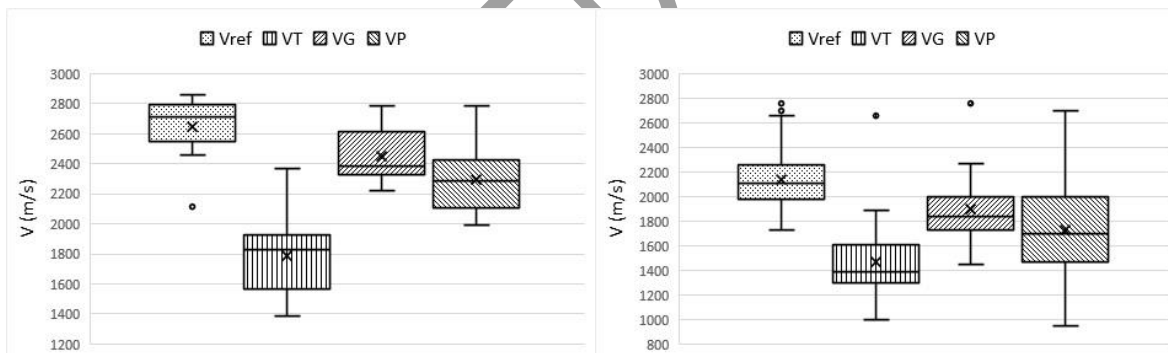
396 Table 6 reports the propagation velocity V in correspondence with the various defects as a
 397 function of the type of reinforcement, whereas Table 7 reports the results of UPV carried out
 398 on strengthened specimens with the artificial adhesion defects, along with V of the
 399 strengthened specimens with perfect adhesion assumed as reference. In view of the results
 400 previously commented (Table 5), the fact that different types of reinforcement were applied
 401 was not considered in Table 7 and subsequent analyses.

402 Table 6. Velocity of propagation along defective paths. V_m = average V, DS = standard deviation, CV = coeff.
 403 of variation.

	FRP lamina						FRP sheet						FRCM sheet					
	Teflon		Glue		Plastic		Teflon		Glue		Plastic		Teflon		Glue		Plastic	
	DT	ST	DT	ST	DT	ST	DT	ST	DT	ST	DT	ST	DT	ST	DT	ST	DT	ST
V _m (m/s)	1758	1360	2456	1766	2324	1718	1857	1406	2571	1856	2360	1716	1760	1652	2331	2071	2209	1915
DS (m/s)	299	178	135	156	278	286	339	146	166	149	199	175	151	381	98	295	139	392
CV	0.170	0.131	0.055	0.088	0.120	0.166	0.182	0.104	0.065	0.080	0.084	0.102	0.086	0.231	0.042	0.143	0.063	0.205

404 Table 7. Velocity of propagation in reference specimens and in defective specimens. s = number of specimens, n
405 = total number of measurements, V_m = average V, DS = standard deviation, CV = coeff. of variation, ΔV = V_m
406 decrease due to defects. The strengthening type is not considered.

	Reference specimens without defects		Adhesion defect: Teflon		Adhesion defect: Glue		Adhesion defect: Plastic	
	DT	ST	DT	ST	DT	ST	DT	ST
s	6	6	24	24	24	24	24	24
n	18	36	24	48	24	48	24	48
V _m (m/s)	2650	2140	1792	1502	2453	1905	2297	1763
DS (m/s)	214	245	261	305	161	264	209	360
CV	0.081	0.114	0.146	0.203	0.065	0.138	0.091	0.204
ΔV (%)	-	-	-32.38	-27.79	-7.43	-10.97	-13.30	-17.61



407 Figure 14. V (m/s) data in specimens with adhesion defects. DT (left) and ST (right). V_{ref} = V in reference
408 specimens without defects, V_T = V in correspondence with Teflon, V_G = V in correspondence with Glue, V_P =
409 V in correspondence with Plastic.
410

411 From Tables 6 and 7 and from the plots in Figure 14, the presence of defects determines, with
412 respect to the reference specimens without defects, a decrease in V different according to the
413 type of defect considered. This is confirmed by the application of the Mann-Whitney U test,
414 reported in Tables 8 and 9, which gives the probability of rejecting the null hypothesis.

415 Table 8. Probability of rejecting the null hypothesis of Mann-Whitney U test for V difference between non-
416 defective and defective specimens. Significance level p = 0.05. U is compared to critical value U* at p < 0.05.
417 U* = 216 for DT measurements and = 864 for ST measurements.

	V _{ref} vs V _T		V _{ref} vs V _G		V _{ref} vs V _P	
	DT	ST	DT	ST	DT	ST

Mann-Whitney U test	5.5<216	57<864	88.5<216	387.5<864	54<216	307<874
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418 Table 9. Probability of rejecting the null hypothesis of Mann-Whitney U test for V difference between paths
419 running into different defects. Significance level $p = 0.05$. U is compared to critical value U^* at $p < 0.05$. $U^* =$
420 288 for DT measurements and = 1152 for ST measurements.

	VT vs VG		VT vs VP		VG vs VP	
	DT	ST	DT	ST	DT	ST
Mann-Whitney U test	12<288	256<1152	41<288	593.5<1152	157.5<288	798.5<1152

421 The results reported in Table 8 signify that V data associated with defective and non-defective
422 specimens are extracted from different populations with a probability higher than 95%.
423 Similarly, the results in Table 9 signify that V data associated with paths running into different
424 defects are extracted from different populations with a probability higher than 95%. It is
425 interesting to note that Teflon causes the greatest V decrease - about 32% for DT and 28% for
426 ST -, followed by the plastic - about 13% for DT and 18% for ST -, and finally by the glue,
427 which instead determines the minor decrease - about 7% for DT and 11% for ST (see Table 6).
428 This outcome is consistent with the different acoustic impedance of the materials used to
429 simulate the defects. In the most common databases [54] it is reported that Teflon has a lower
430 acoustic impedance than the other materials involved in the experiment, and for this reason it
431 is frequently used for the simulation of adhesion defects at the interface between FRC and
432 substrate. The sensitivity of UPV, both in DT and ST modes, to the different type of defect is
433 a result worthy of further investigation, as the technique could be used both to identify the
434 presence of adhesion defects and to discriminate the type of defect.

435 5. Limits of application

436 The results previously explained point out that UPV allows to identify the presence of adhesion
437 defects and evoke the possibility of discriminating between different defects both when using
438 DT and ST. Therefore, also the ST method is effective in this type of investigation, and this is
439 especially interesting since the DT often cannot be used due to the impossibility of accessing
440 opposite surfaces of the structural element.

441 However, there are some aspects, listed below, that need to be further explored to confirm the
442 effectiveness of the proposed approach, especially in on-site applications.

443 In the herein experiment, the acquisitions of ultrasonic signals are made by setting the emitter
444 transducer in correspondence with the defect, whose position is known. On the one hand, this
445 allows to evaluate the effectiveness of the methodology used, on the other, however, it does
446 not necessarily reflect the real situations, in which indeed the position of any defect is not
447 known, but is instead the unknown to be detected. Therefore, it would be appropriate, to test
448 the effectiveness of UPV in on-site investigations, to carry out an experimental campaign in
449 which the position of the transducers is random with respect to the position of the defects. This
450 can be done, for example, by defining a proper grid of measuring points randomly arranged
451 and evaluating the sensitivity of DT and ST in this situation. It should be taken into account
452 that the choice of the measurements grid significantly influences the diagnostic capacity of
453 UPV, and that the proportion between the grid pitch and the size of the defect, along with the

454 misalignment between the centre of the grid cell and the position of the defect, are very
455 influencing factors [55].

456 As with all NDT, there are limits beyond which defects cannot be identified by UPV.
457 Generally, these limits depend on the characteristics of the signal (frequency and wavelength).
458 In fact, as a rule, defects smaller than the wavelength of the signal are scarcely or not at all
459 detectable, so high-frequency signals are more sensitive, but less penetrating due to the lower
460 energy, than low-frequency signals. A choice should therefore be made concerning transducers
461 and signals according to the size of the defects to be detected. In the specific case of the
462 adhesion defects between FRC and concrete, to simplify, the problem translates into the
463 definition of the dimensional threshold below which the adhesion defect is considered
464 irrelevant. This very important aspect is still being studied and debated, is addressed in some
465 standards and guidelines [56] and is however outside the scope of this work. In the event of
466 using commercial-type ultrasound equipment, which has piezoelectric transducers with
467 frequencies generally varying between 20 kHz and 500 kHz, it is necessary to establish what
468 is the velocity decrease threshold above which the presence of a defect is supposed. The use of
469 V measurement grids can be useful also for this purpose since they allow the processing of V
470 maps where to identify the position of areas with low V values. The threshold for supposing a
471 defect can be roughly defined by the standard deviation from the average V of the various
472 points of the grid. However, this is certainly an aspect worthy of further study.

473 **6. Conclusions**

474 The results of Ultrasonic Pulse Velocity testing application to detect adhesion defects at the
475 interface between FRP/FRCM and concrete have been reported and discussed. Tests have been
476 carried out on several strengthened concrete specimens in which some artificial adhesion
477 defects have been specifically settled. Both direct and semi-direct ultrasonic measurements
478 have been applied, being the propagation velocity of the ultrasonic signal the analysed quantity.
479 The conclusions can be summarized as follows.

- 480 • The velocities of propagation measured with the direct and the semi-direct techniques are
481 appreciably different, with percentage difference in non-defective specimens varying in the
482 range 18%-30% depending on the presence of reinforcement or not. This agrees with the
483 theoretical model that generally acknowledges higher travel time for paths different from
484 the one along the axis of the ultrasonic beam.
- 485 • The three types of reinforcements (FRP lamina, FRP sheet, and FRCM sheet) do not have
486 an appreciably different effect on velocity measurements in the context of each
487 measurement method (DT or ST), as highlighted by the U test which discards the
488 hypothesis that the three samples are extracted from one single population with a
489 probability of about 95%.
- 490 • The presence of adhesion defects determines, with respect to the reference specimens
491 without defects, a decrease in velocity significantly different, for both direct and semi-
492 direct modes, according to the type of defect considered. Teflon causes the greatest V
493 decrease, about 32% for DT and 28% for ST, followed by the plastic, about 13% for DT
494 and 18% for ST, and finally by the glue, about 7% for DT and 11% for ST. This is consistent
495 with the different acoustic impedances of the materials involved in the experimental test.
- 496 • The semi-direct transmission is a testing method as fitting as the direct one in detecting the
497 adhesion defects, and this is particularly interesting since in many real situations the
498 application of direct transmission is prevented.

499 Further research would be appropriate to confirm the effectiveness of the proposed approach
500 on site, considering the influence of the following important issues.

- 501 • The position of the transducers with respect to the defect, perhaps evaluating the sensitivity
502 of DT and ST when a grid of measuring points randomly arranged is settled.
- 503 • The size of the defect, whose detectability is affected by the characteristics of the signal,
504 the measurements points arrangements, and the misalignment between the signal path and
505 the position of the defect.
- 506 • The velocity threshold that identifies a defect, which could be roughly defined as the
507 standard deviation from the average V of the various signal paths.

508

509 **Conflicts of Interest**

510 The authors declare that there is no conflict of interest regarding the publication of this paper.

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